

**“THRUST BLOCKS AS A MEANS OF  
LATERAL SUPPORT FOR EXCAVATIONS”.  
FROM PREDICTION TO REALITY**

Marc Cabrera

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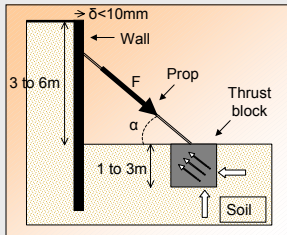
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**THE GEOTECHNICAL PROBLEM**

- Response of shallow embedded foundations under equivalent prop loading
- The stiffness provided by raking props and thrust blocks when small displacements are permitted
- The complex soil-structure interaction
- Establish clear guidelines for the use of thrust blocks in a variety of ground conditions



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**METHODOLOGY**

- MODELLING
  - Centrifuge Modelling
  - FEM
- PRACTICE
  - Monitoring field data
  - Field test

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## WORK TO DATE

- 12 Centrifuge test in sands
- Design of the field test

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## CENTRIFUGE MODELLING



Acutronic 661 Geotechnical Centrifuge

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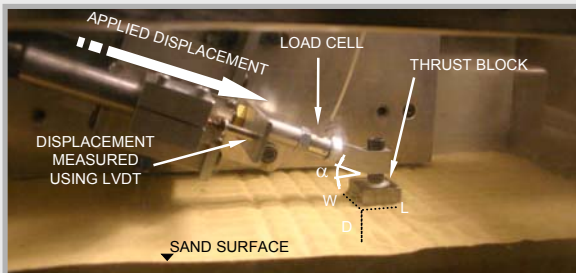
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## CENTRIFUGE MODELLING

- The Test Configuration



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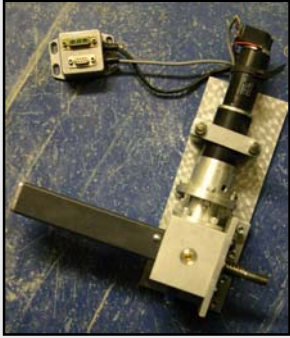
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# CENTRIFUGE MODELLING

• The Actuator

• The Displacement Transfer Connection



LEAD SCREW

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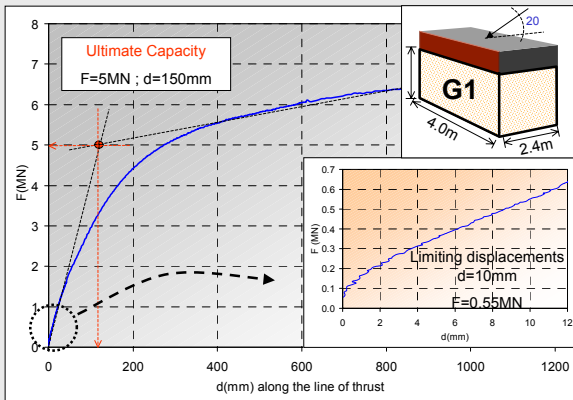
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# GENERAL RESULTS




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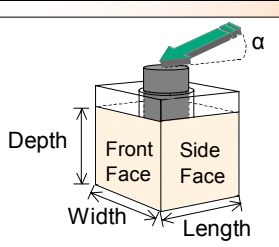
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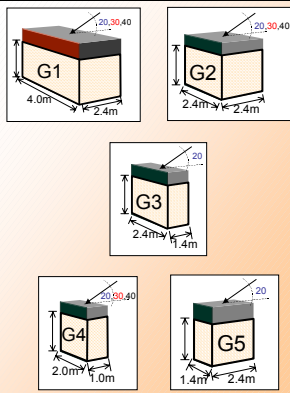
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# TEST CARRIED OUT



- CONSTANT DEPTH 2.4m
- Prop angles used:  $20^\circ$ ,  $30^\circ$ ,  $40^\circ$




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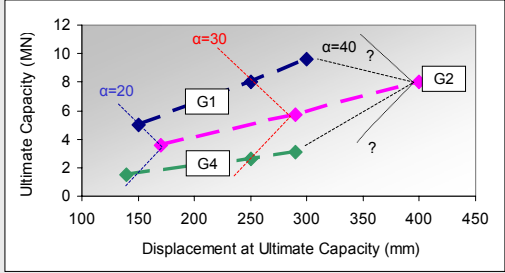
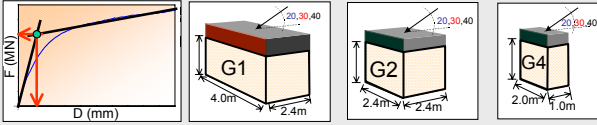
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# ANALYSIS AT ULTIMATE CAPACITY




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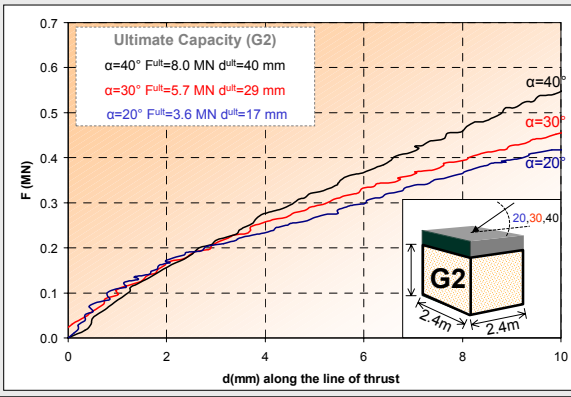
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# ANALYSIS LIMITING DISPLACEMENTS




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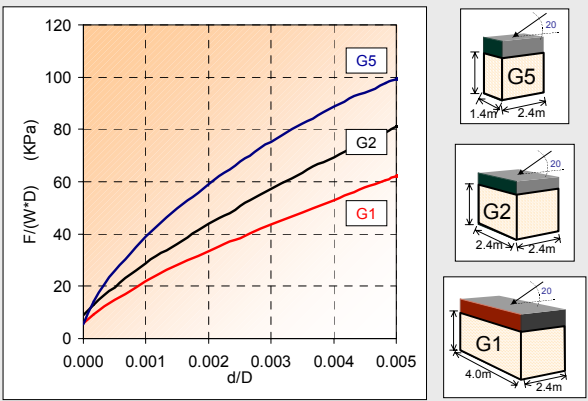
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# ANALYSIS OF WIDTH INFLUENCE




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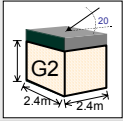
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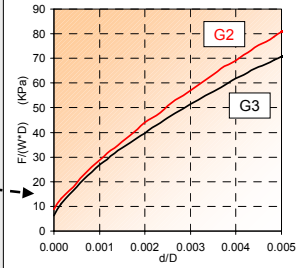
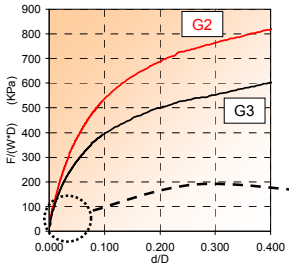
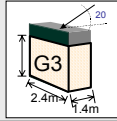
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# ANALYSIS OF LENGTH INFLUENCE



$\alpha=20^\circ$  W=2.4m




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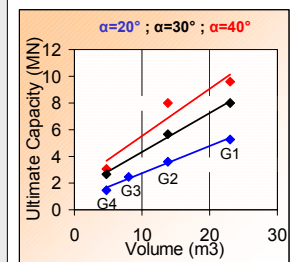
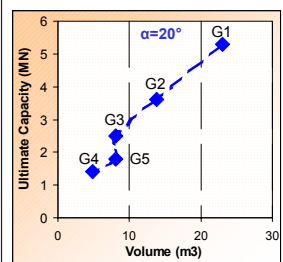
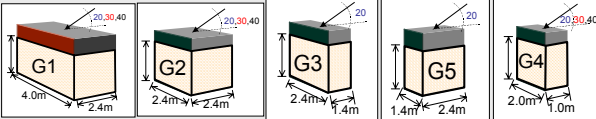
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# ULTIMATE CAPACITY




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# SUMMARY OF CENTRIFUGE TESTS

## • At ultimate capacity

Force

Displacement

- increases with prop angle
- depends of the volume of the thrust block .
- increases with prop angle
- there is no simple relationship with the geometry of the thrust block.

## • For prop angle of 20°

- The 3-D effects are important, at initial displacements.
- The area of the base face is important at large displacements.

- For displacements < 10mm the thrust block capacity is governed by width and depth.

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# FIELD TEST LOCATION

- 214-219 OXFORD STREET, LONDON.
- KELTBRAY



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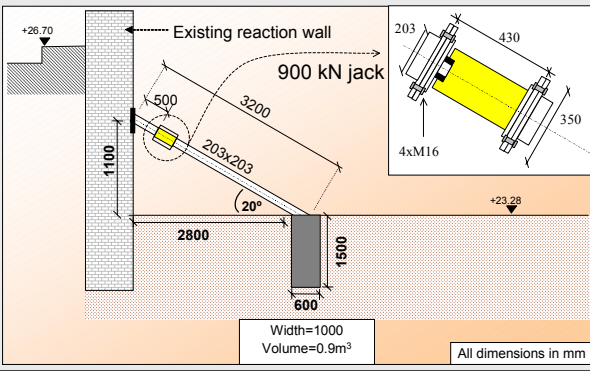
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# DESIGN OF THE FIELD TEST



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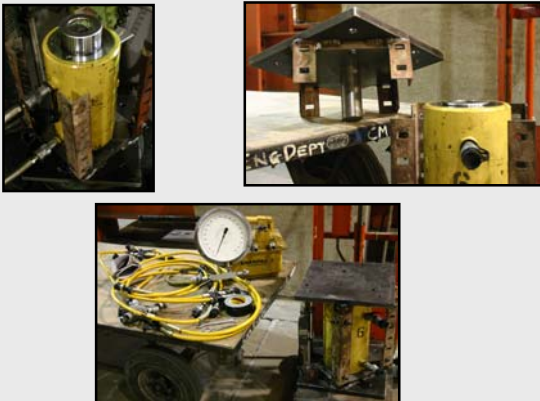
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# MATERIAL FOR THE FIELD TEST



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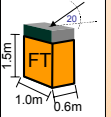
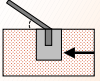
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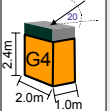
# THE FIELD TEST RESULTS EXPECTED

Plane Strain Upper Bound Method

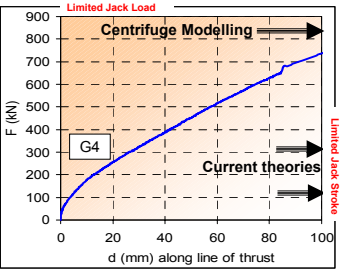

$$F_p = \frac{1}{2} \cdot \gamma \cdot W \cdot H^2 \cdot K_p$$
$$K_p = f(\Phi; \delta / \Phi; \lambda; \beta)$$

- $\Phi = 32 \text{deg}$
- $\gamma = 18 \text{KN/m}^3$
- $B = 20^\circ$
- $\delta / \Phi = 1$

$F_{p, \delta=1} = 100 \text{ to } 300 \text{ kN}$



Ultimate Capacity  
 $\alpha = 20^\circ$   
G4  $F = 1400 \text{ kN}$   $\delta^{ult} = 146 \text{ mm}$   
FT  $F = 900 \text{ kN}$   $\delta^{ult} = ?$



Limited Jack Load

Centrifuge Modelling

Current theories

Limited Jack Stroke

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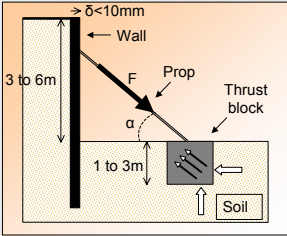
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# CONCLUSIONS



- Some realistic magnitudes of prop axial loads acting as a lateral support for excavations.
- Relative contribution of front and base face.
- Some relations between volume of concrete and maximum load reach.
- Some realistic magnitudes of the stiffness needed to reach the desired load.

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# INDUSTRY COLLABORATORS

- Keltbray
- Wates Group Ltd
- Ove Arup and Partners
- Sir Robert McAlpine Ltd
- Wentworth House Partnership

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